

Modified Adomian decomposition method for fracture of laminated uni-directional composites

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Abstract. In this paper, the well-known Adomian Decomposition Method (ADM) is modified to solve the fracture laminated multi-directional problems. The results are compared with the existing analytical/exact or experimental method. The already known existing ADM is modified to improve the accuracy and convergence. Thus, the modified method is named as Modified Adomian Decomposition Method (MADM). The results from MADM are found to converge very quickly, simple to apply for fracture(singularity) problems and are more accurate compared to experimental and analytical methods. MADM is quite efficient and is practically well-suited for use in these problems. Several examples are given to check the reliability of the present method. In the present paper, the principle of the decomposition method is described, and its advantages form the analyses of fracture of laminated uni-directional composites.

Keywords. Fracture of laminated uni-directional composites; initial value problems; modified Adomian decomposition method; experimental and analytical solutions.

1. Literature review on laminated fracture

It is surprising that the Adomian Decomposition Method (ADM) has not been applied for ordinary differential equations appearing in the fracture problem. Particularly in laminated composites, (we solved) several coupled ordinary differential equations and it appears from literature that ADM has not been applied at all. In further review, focus has been placed on the analytical and computational techniques to analyse end notched flexure (ENF) and end notched cantilever (ENC) specimens for inter-laminar fracture toughness of composites. Carlsson & Gillespie (1989) reviewed the fracture mechanics and experimental mechanics approaches used to characterize mode II inter-laminar fracture of composites. The review covers the detailed technical aspects of analytical, numerical and experimental methods to analyse ENF specimen for the mode II inter-laminar fracture toughness.

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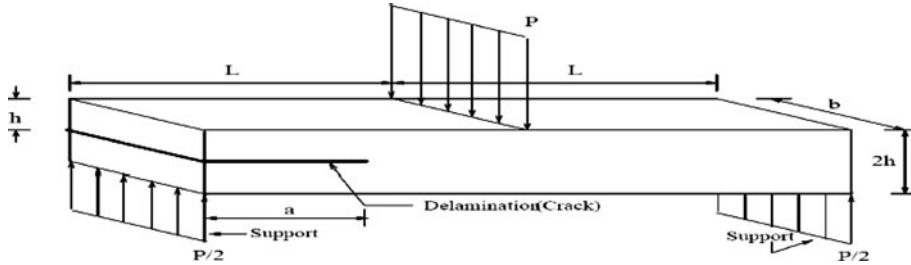


Figure 1. A schematic ENF test specimen.

1.1 Mode II: ENF & ENC specimens

The short beam shear (SBS) test method was perhaps the first test method used to investigate inter-laminar fracture of composite laminates on a routine basis (Whitney *et al* 1974; Whitney & Browning 1998; Whitney 1991). Introduced in the 1960s, the method still remains an important quality control test. This test method, however, has some limitations. The small span to depth ratio, in conjunction with perturbations in the stress distribution due to load introduction that do not decay rapidly in orthotropic materials, violates the beam theory based data-reduction scheme and promotes alternate failure models by Whitney & Browning (1998), Whitney (1991). Consequently, the SBS test method measures the apparent inter-laminar shear strength of the composite. Classical linear elastic fracture mechanics (LEFM) has been used more recently to characterize the inter-laminar fracture toughness of composite materials. In contrast to the SBS test method, well-defined delaminations are embedded or machined into the specimen. Barrett & Foschi (1977) utilized ENF specimen to characterize the mode II inter-laminar fracture of cracked wood beams. Russell & Street (1982) used this specimen to characterize mode II critical strain energy release rates of advanced composites. The geometry of the ENF specimen is essentially a three-point flexure specimen with an embedded through-width delamination placed at the laminate mid-surface. The delamination is placed at the end of the specimen to accommodate the sliding deformation of the sub-laminates that result from the flexural loading. A typical ENF specimen is 25 mm wide (w), 100 mm long ($2L$) and 3–4 mm in thickness ($2h$). Carlsson *et al* (1986b) presented an analysis of ENF specimen for the characterization of mode II inter-laminar fracture toughness. Shear deformation plate theory mentioned in Whitney & Pagano (1970), which incorporates a shear stress singularity at the crack tip is employed and correlated with simple beam theory expressions and finite element results for compliance and strain energy release rate of the specimen. The shear deformation plate theory solution indicates that the strain energy release rate is highly sensitive to the characteristic distance associated with the decay of the shear stress singularity to beam theory behaviour in the ENF geometry. Parametric study has been carried out to investigate the influence of geometry and material properties on compliance and strain energy release rate (figure 1 and figure 4).

2. Stress analysis models and concepts

The analyses of unidirectional end notched flexure and end notched cantilever specimens, using first order shear deformation beam theory (FOBT) and second order shear deformation beam theory (SOBT) theories, have been presented for mode II (shearing mode) inter-laminar fracture toughness of composites, following the method adopted in the paper by

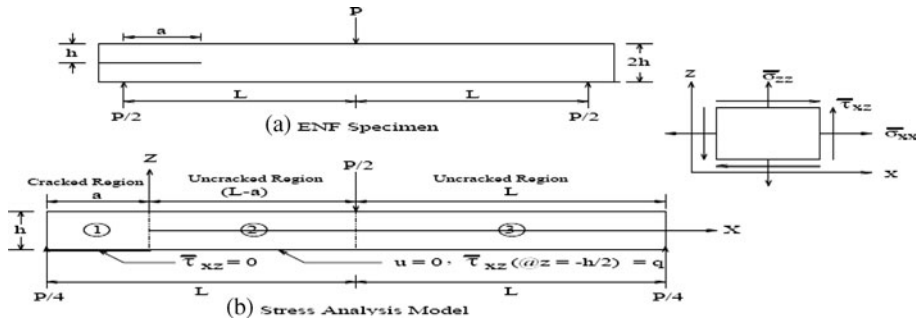


Figure 2. ENF specimen and its stress analysis mode.

Raghu Prasad & Pavankumar (2008), Pavankumar & Raghu Prasad (2003, 2008, 2009), Pavankumar (2004). No credit is claimed for the variational method adopted to derive the governing differential equation and also the idea of matching conditions because they are exactly same as the method in reference mentioned above. This contribution is trying to obtain a more effective solution through the method developed in the present paper viz. MADM. Therefore, in order to prove the effectiveness of MADM the same examples with the same nomenclature in the reference are solved by MADM. Therefore, several equations like those for boundary conditions and matching conditions are repeated and they appear in references mentioned. The compliance and strain energy release rate (SERR), obtained from the present paper, have been compared with the existing experimental, analytical and finite element results in the literature. The unidirectional ENF and ENC specimens considered for the analysis have been shown in figures 2a and 3a. The respective stress analysis models have been shown in figures 2b and 3b and these are similar to the stress analysis models proposed by Whitney (1990). These stress analysis models consider only upper halves i.e., above delamination plane of ENF and ENC specimens because of the fact that delamination is at mid-plane and lamination scheme is symmetric about the mid-plane of ENF and ENC specimens. The stress analysis models of ENF and ENC specimens have cracked and uncracked regions. In the uncracked region, at the bottom of the stress analysis models i.e., at the mid-plane of the actual ENF and ENC specimens, surface traction ‘q’ exists and axial displacement u is zero. Further, it has been assumed that the delaminated faces slide over each other freely which means that the frictional effects between the delaminated faces have been neglected. The appropriate governing differential equations, for

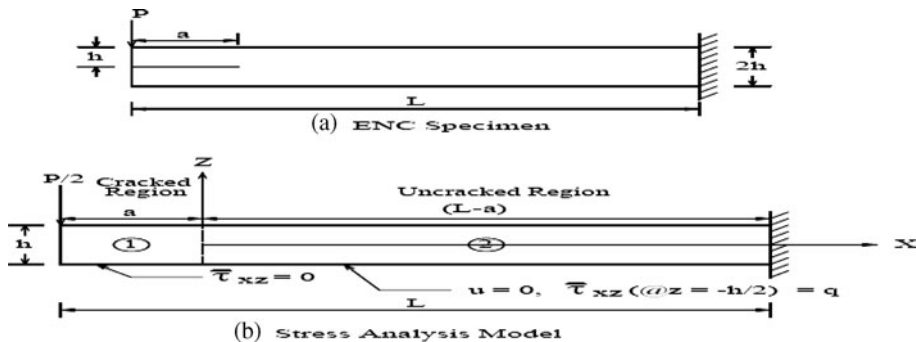


Figure 3. ENC specimen and its stress analysis mode.

cracked and uncracked regions of the ENF and ENC specimens, can be written based on FOBT and SOBT which have been derived using MADM. Then the governing differential equations of cracked and uncracked regions can be solved systematically. The solution consists of integration constants which can be determined from boundary and matching conditions. ‘Special attention is necessary for continuity or matching conditions at the crack tip. In the present paper, appropriate matching conditions, in terms of generalized displacements and stress resultants, have been applied at the crack tip by enforcing displacement continuity at the crack tip in conjunction with variational equation’ (2003, 2008, 2009). Once the solution is obtained, deflection under the load can be determined using appropriate expressions. Further, compliance can be determined from the deflection under the load and strain energy release rate (SERR) can be calculated using MADM or numerical approach. It should be noted that the present solutions of ENF and ENC specimens are perfectly compatible with deformations that occur in the lower halves of the ENF and ENC specimens.

2.1 Mathematical model

Again, here the mathematical model is borrowed from the earlier work by Raghu Prasad & Pavankumar (2008), Pavankumar & Raghu Prasad (2003, 2008, 2009), Pavankumar (2004). In the stress analysis mode of ENF specimen, one can have two regions viz., cracked [$-a \leq x \leq 0$] and uncracked [$0 \leq x \leq (2L - a)$] regions. The regions [$-a \leq x \leq 0$], [$0 \leq x \leq (L - a)$] and [$(L - a) \leq x \leq (2L - a)$] have been idealized as three different beams 1, 2 and 3, respectively with imaginary cuts at the crack tip and at the point of load application. To get appropriate governing differential equations, based on various laminated composite beam theories, for the analysis of cracked and uncracked regions of ENF and ENC specimens, the following substitutions need to be done in the equations.

2.1a Cracked region:

$$\bar{\tau}_{xz} \left(\text{at } z = \pm \frac{h}{2} \right) = 0 \quad (1a)$$

$$\bar{\sigma}_{zz} \left(\text{at } z = \pm \frac{h}{2} \right) = 0. \quad (1b)$$

2.1b Uncracked region:

$$\bar{\tau}_{xz} \left(\text{at } z = -\frac{h}{2} \right) = q \quad (2a)$$

$$\bar{\tau}_{xz} \left(\text{at } z = \frac{h}{2} \right) = 0 \quad (2b)$$

$$\bar{\sigma}_{zz} \left(\text{at } z = \pm \frac{h}{2} \right) = 0. \quad (2c)$$

2.2 MADM procedure for ENF and ENC specimens using first order shear deformation beam theory

2.2a *Governing differential equations for cracked and uncracked regions:* The equilibrium equations for uncracked region, based on FOBT, can be obtained by using equation (2) as in the

equations, and the equilibrium equations for uncracked region, based on FOBT, can be obtained by Raghu Prasad & Pavankumar (2008), Pavankumar & Raghu Prasad (2003, 2008, 2009), Pavankumar (2004) as

$$\frac{dN_{xx}}{dx} - bq = 0 \Rightarrow L_x N_{xx} - bq = 0 \quad (3a)$$

$$\frac{dQ_{xz}}{dx} = 0 \Rightarrow L_x Q_{xz} = 0 \quad (3b)$$

$$\frac{dM_{xx}}{dx} - Q_{xz} + \frac{bh}{2}q = 0 \Rightarrow L_x M_{xx} - Q_{xz} + \frac{bh}{2}q = 0, \quad (3c)$$

where linear operator is $L_x = \frac{d}{dx}$. Further, for the cracked region, equilibrium equations can be obtained from the above equation (3) by putting $q = 0$ i.e., delaminated faces slide freely over each other.

2.2b Inter-laminar shear stress resultant expressions for cracked and uncracked regions: It is possible to consider two choices of inter-laminar shear stress resultant expressions for cracked and uncracked regions and they are as follows, it may also be noted here that MADM solution for CBT is not obtained because Q_{xz} becomes zero.

FOBT¹: For both cracked and uncracked regions, inter-laminar shear stress resultants, have been considered with shear correction factor $k_1^2 = 1$. *Choosing shear correction factor as unity is equivalent to saying that shear correction factor is not introduced into the formulation.* This choice has been named as FOBT¹.

FOBT²: For both cracked and uncracked regions, inter-laminar shear stress resultant, have been considered with the assumed shear correction factor $k_1^2 = \frac{5}{6}$. This choice has been named as FOBT².

Beside SOBT² as explained above as one other solution classified as SOBT¹ is available in the work of Raghu Prasad & Pavankumar (2008), Pavankumar & Raghu Prasad (2003, 2008, 2009), Pavankumar (2004). MADM solutions are obtained for SOBT¹ with shear correction factors k_1^2 . It may also be noted here that MADM solution for CBT is not obtained because Q_{xz} becomes zero.

$$Q_{xz} = k_1^2 G bh \left(\frac{dw_o}{dx} + \psi_x \right) + f_q bhq \quad (4a)$$

$$= k_1^2 G bh (L_x w_o + \psi_x) + f_q bhq \quad (4b)$$

and the quantities k_1^2 and f_q will take the values as given in the following table depending upon cracked or uncracked regions. The MADM solution for $k_1^2 = 1$ and $\frac{5}{6}$ does not show much difference in FOBT^{1,2}, therefore FOBT^E is represented as FOBT. Similarly, FOBT and CBT MADM results are closure. For CBT $\psi_x = -L_x w_o = -\frac{dw_o}{dx}$ and the above equation (4) becomes $Q_{xz} = f_q bhq$.

2.2c MADM Solution: Cracked Region $[-a \leq x \leq 0]$ – Beam 1 (figures 2b and 3b):

L_x^{-1} of first and second of equation (3) (with $q = 0$) gives

$$L_x N_{xx} = 0 \Rightarrow N_{xx} = C_1 \quad (5a)$$

$$L_x Q_{xz} = 0 \Rightarrow Q_{xz} = C_2, \quad (5b)$$

L_x^{-1} of resulting equation, obtained from the substitution of equation (5) in the third of equation (3) (with $q = 0$), gives

$$L_x M_{xx} = Q_{xz} \Rightarrow M_{xx} = C_2 x + C_3. \quad (6)$$

By using stress resultant expressions in equation (4) in equations (1), (3) and (5) (with $q = 0$), one can get the following equilibrium equations in terms of generalized displacements,

$$\bar{E}bh \frac{du_o}{dx} = C_1, \Rightarrow \bar{E}bh L_x u_o = C_1, \Rightarrow u_o = C_4 + L_x^{-1} \frac{C_1}{\bar{E}bh} \quad (7a)$$

$$Gbhk_1^2 \left(\frac{dw_o}{dx} + \psi_x \right) = C_2, \Rightarrow Gbhk_1^2 (L_x w_o + \psi_x) = C_2, \quad (7b)$$

$$w_o = C_5 + L_x^{-1} \frac{C_2}{Gbhk_1^2} - L_x^{-1} \psi_x$$

$$\frac{\bar{E}bh^3}{12} \frac{d\psi_x}{dx} = C_2 x + C_3, \quad L_x \psi_x = \frac{C_2 x + C_3}{EI}, \quad (7c)$$

$$\psi_x = C_6 + L_x^{-1} \frac{12(C_2 x + C_3)}{\bar{E}bh^3}.$$

From the above equations (5)–(7), one can re-write using MADM $u_o = C_4 + L_x^{-1} \frac{C_1}{\bar{E}bh}$ and $(L_x w_o + \psi_x) = \frac{C_2}{Gbhk_1^2}$ from this equation $\psi_x = \frac{C_2}{Gbhk_1^2} - L_x w_o = C_5 + \frac{6C_2 x^2 + 12C_3 x}{\bar{E}bh^3}$ and using MADM one can find, $L_x w_o = \frac{C_2}{Gbhk_1^2} - \left(C_5 + \frac{6C_2 x^2 + 12C_3 x}{\bar{E}bh^3} \right)$, it can be solved easily. The solution for cracked region [$-a \leq x \leq 0$] (beam 1) can be summarized as

$$u_o = \frac{C_1 x}{\bar{E}_{11}bh} + C_4 \quad (8a)$$

$$w_o = \frac{C_2}{\bar{E}_{11}b} \left(\frac{1}{k_1^2} \frac{\bar{E}_{11}}{G_{13}} \frac{x}{h} - 2 \frac{x^3}{h^3} \right) - 6 \frac{C_3}{\bar{E}_{11}bh} \frac{x^2}{h^2} - C_5 h \frac{x}{h} + C_6 \quad (8b)$$

$$\psi_x = 6 \frac{C_2}{\bar{E}_{11}bh} \frac{x^2}{h^2} + 12 \frac{C_3}{\bar{E}_{11}bh^2} \frac{x}{h} + C_5. \quad (8c)$$

From these generalized displacement expressions, corresponding stress resultant expressions can be obtained easily by using the above equations.

Uncracked Region ($0 \leq x \leq (2L - a)$):

In the uncracked region i.e., beams 2 and 3, $q \neq 0$. Further, axial displacement along $z = -\frac{h}{2}$ has been assumed to be zero. This can be written as $u(z = -\frac{h}{2}) = 0$. By using the above equation (1), u_o can be expressed in terms of ψ_x as, $u_o = \frac{h}{2} \psi_x$ and now the surface traction ‘ q ’ is the unknown. Hence, in-plane stress resultant expression of N_{xx} will be modified, by using the above relation,

$$N_{xx} = \frac{\bar{E}_{11}bh^2}{2} \frac{d\psi_x}{dx}. \quad (9)$$

Further, L_x^{-1} of second of equation (3) gives,

$$Q_{xz} = \bar{C}_1. \quad (10)$$

By substituting the modified in-plane stress resultant expression N_{xx} as in the above equation (9), stress resultant expression of M_{xx} and inter-laminar shear stress resultant expression of equation (10), first and third equations of equation (3), one can get the following equilibrium equations in terms of generalized displacements and surface traction 'q' for uncracked region.

$$\frac{d^2 \psi_x}{dx^2} - \frac{2}{\bar{E}_{11} h^2} q = 0 \Rightarrow L_{2x} \psi_x - \frac{2}{\bar{E}_{11} h^2} q = 0 \quad (11a)$$

$$\begin{aligned} \left(\frac{dw_o}{dx} + \psi_x \right) + \frac{f_q}{G_{13} k_1^2} q &= \frac{\bar{C}_1}{G_{13} k_1^2 bh} \\ \Rightarrow (L_x w_o + \psi_x) + \frac{f_q}{G_{13} k_1^2} q &= \frac{\bar{C}_1}{G_{13} k_1^2 bh} \end{aligned} \quad (11b)$$

$$\frac{d^2 \psi_x}{dx^2} + \frac{6q}{\bar{E}_{11} h^2} = \frac{12\bar{C}_1}{\bar{E}_{11} bh^3} \Rightarrow L_{2x} \psi_x + \frac{6q}{\bar{E}_{11} h^2} = \frac{12\bar{C}_1}{\bar{E}_{11} bh^3}, \quad (11c)$$

where linear operator $L_{2x} = \frac{d^2}{dx^2}$. From the first of equations (11), one can re-write MADM form as, $Q_{xz} = C_2 = G_{13} bh k_1^2 (L_x w_o + \psi_x)$ or $(L_x w_o + \psi_x) = \frac{C_2}{G_{13} bh k_1^2}$, also can know the relation $N_{xx} = C_1 + bq x = \bar{E}_{11} bh L_x u_o$, $u_o = \frac{bq \frac{x^2}{2} + C_1 x}{\bar{E}_{11} bh} + C_4$ with known u_o , ψ_x can be obtained as (u_o at $x = \frac{-h}{2}$) ψ_x as $\psi_x = \left(C_5 + \frac{-12C_1 x^2 + 6C_2 x^2 + 12C_3 x + 24C_4 \bar{E}_{13} b x^2}{\bar{E}_{13} bh^3} \right) + \frac{f_q}{G_{13} k_1^2}$ and $\frac{dw_o}{dx} = \frac{C_2}{G_{13} bh k_1^2} - \psi_x + \frac{f_q}{G_{13} k_1^2}$ it can be solved easily. Substitution of the above equations in the second equation (11) yields

$$w_o = \frac{\bar{C}_1}{\bar{E}_{11} b} \left\{ \left(4 \frac{x^2}{h^2} - \frac{3}{2} f_q \right) \frac{\bar{E}_{11}}{G_{13}} \frac{1}{k_1^2} \frac{x}{h} - \frac{1}{2} \frac{x^3}{h^3} \right\} - \bar{C}_2 \frac{x^2}{2} - \bar{C}_3 x + \bar{C}_4. \quad (12)$$

For the uncracked region ($0 \leq x \leq (L - a)$) (beam 2), solution can be summarized as

$$w_o = \frac{\bar{C}_1}{\bar{E}_{11} b} \left\{ \left(4 \frac{x^2}{h^2} - \frac{3}{2} f_q \right) \frac{\bar{E}_{11}}{G_{13}} \frac{1}{k_1^2} \frac{x}{h} - \frac{1}{2} \frac{x^3}{h^3} \right\} - \bar{C}_2 \frac{x^2}{2} - \bar{C}_3 x + \bar{C}_4 \quad (13a)$$

$$q = \frac{4C_1}{bh} - \frac{8\bar{E}C_4}{h}. \quad (13b)$$

Similar solution can be obtained for uncracked region ($(L - a) \leq x \leq (2L - a)$) (beam 3) and the solution can be written, with different integration constants as

$$w_o = \frac{\tilde{C}_1}{\bar{E}_{11} b} \left\{ \left(4 \frac{x^2}{h^2} - \frac{3}{2} f_q \right) \frac{\bar{E}_{11}}{G_{13}} \frac{1}{k_1^2} \frac{x}{h} - \frac{1}{2} \frac{x^3}{h^3} \right\} - \tilde{C}_2 \frac{x^2}{2} - \tilde{C}_3 x + \tilde{C}_4 \quad (14a)$$

$$q = \frac{4C_1}{bh} - \frac{8\bar{E}C_4}{h}. \quad (14b)$$

From these generalized displacement expressions, corresponding stress resultant expressions can be obtained easily by using equations (14a–14b). According to FOBT, for ENF and ENC specimens, the unknown constants have to be determined from boundary and matching conditions given in the sections 2.2 and 2.3 for ENF and ENC specimens, respectively. Application of these boundary and matching conditions results in simultaneous equations for ENF and ENC specimens. These simultaneous equations have been solved using MADM for the unknown constants. Once these constants are determined, deflection under the load at $x = (L - a)$ for ENF specimen and at $x = -a$ for ENC specimen has been determined using equations for ENF and ENC specimens, respectively. From the deflection thus obtained, compliance and then SERR have been obtained using the procedures presented in section 3.

2.3 MADM Solution for ENF and ENC specimens using second order shear deformation beam theory

2.3a *Governing differential equations for cracked and uncracked regions:* The equilibrium equations for uncracked region, based on SOBT, can be obtained as given below

$$\frac{dN_{xx}}{dx} - bq = 0 \Rightarrow L_x N_{xx} - bq = 0 \quad (15a)$$

$$\frac{dQ_{xz}}{dx} = 0 \Rightarrow L_x Q_{xz} = 0 \quad (15b)$$

$$\frac{dM_{xx}}{dx} - Q_{xz} + \frac{bh}{2}q = 0 \Rightarrow L_x M_{xx} - Q_{xz} + \frac{bh}{2}q = 0 \quad (15c)$$

$$\frac{dS_{xx}}{dx} - 2R_{xz} + \frac{bh^2}{4}q = 0 \Rightarrow L_x S_{xx} - 2R_{xz} - \frac{bh^2}{4}q = 0. \quad (15d)$$

Further, for the cracked region, equilibrium equations can be obtained from the above equation (15) by putting $q = 0$ (i.e. delaminated faces slide freely over each other).

2.3b *Inter-laminar shear stress resultants for cracked and uncracked regions:* It is possible to consider two choices of inter-laminar shear stress resultant expressions for cracked and uncracked regions and they are as follows.

SOBT¹: For both cracked and uncracked regions, inter-laminar shear stress resultants given by equations considered with shear correction factors $k_1^2 = 1$ and $k_2^2 = 1$. *Choosing all shear correction factors as unity is equivalent to saying that shear correction factors are not introduced into the formulation.* This choice has been named as SOBT¹.

SOBT²: For both cracked and uncracked regions, inter-laminar shear stress resultants given by equations considered with shear correction factors $k_1^2 = \frac{5}{6}$ and $k_2^2 = \frac{7}{10}$. This choice has been named as SOBT². Inter-laminar shear stress resultant expressions of SOBT^E, SOBT¹ and SOBT² will be written in an unified form as,

$$\begin{aligned} Q_{xz} &= k_1^2 G_{13} bh (L_x w_o + \psi_x) + f_q bhq \\ R_{xz} &= k_2^2 G_{13} \frac{bh^3}{6} \phi_x - f_r bh^2 q \end{aligned} \quad (16)$$

Table 1. Shear correction factors and other quantities for FOBT.

	Cracked region		Uncracked region	
	k_1^2	f_q	k_1^2	f_q
FOBT ^E	5/6	0	5/6	1/12
FOBT ¹	1	0	1	0
FOBT ²	5/6	0	5/6	0

and the quantities k_1^2 , k_2^2 , f_q and f_r will take the values as given in tables 1 and 2 depending upon cracked or uncracked regions.

2.3c MADM Solution:

Cracked Region [$-a \leq x \leq 0$] – *Beam 1* (figures 2b and 3b):

L_x^{-1} of first and second of equation (15) (with $q = 0$) gives

$$N_{xx} = C_1 \quad (17)$$

$$Q_{xz} = C_2, \quad (18)$$

L_x^{-1} of resulting equation, obtained from the substitution of equation (16) in the third of equation (15) (with $q = 0$), gives

$$M_{xx} = C_2x + C_3. \quad (19)$$

By using stress resultant expressions in equation (5) substitute in equations (15)–(18) and in the last equation of equation (15) (with $q = 0$), one can get the following equilibrium equations in terms of generalized displacements,

$$\bar{E}_{11}bh \left(L_x u_o + \frac{h^2}{24} L_x \phi_x \right) = C_1 \quad (20a)$$

$$G_{13}bhk_1^2 (L_x w_o + \psi_x) = C_2 \quad (20b)$$

$$\frac{\bar{E}_{11}bh^3}{12} L_x \psi_x = C_2x + C_3 \quad (20c)$$

$$\frac{\bar{E}_{11}bh^3}{24} \left(L_{2x} u_o + \frac{3h^2}{40} L_{2x} \phi_x \right) k_2^2 G_{13} \frac{bh^2}{12} \phi_x = 0. \quad (20d)$$

Table 2. Shear correction factors and other quantities for SOBT.

	Cracked region				Uncracked region			
	k_1^2	k_2^2	f_q	f_r	k_1^2	k_2^2	f_q	f_r
SOBT ^E	5/6	7/10	0	0	5/6	7/10	1/12	1/40
SOBT ¹	1	1	0	0	1	1	0	0
SOBT ²	5/6	7/10	0	0	5/6	7/10	0	0

From the first of equation (20), one can write

$$L_x u_o = \frac{C_1}{E_{11}bh} - \frac{h^2}{24} L_x \phi_x. \quad (21)$$

L_x^{-1} of above equation gives,

$$u_o = \frac{C_1}{E_{11}bh} x - \frac{h^2}{24} \phi_x + C_4. \quad (22)$$

Using equation (9) in the fourth of equation (20), one can get,

$$\frac{d^2 \phi_x}{dx^2} - \lambda_1^2 h^2 \phi_x = 0 \Rightarrow L_{2x} \phi_x = \lambda_1^2 h^2 \phi_x, \quad (23)$$

where, $\lambda_1 = \sqrt{60k_2^2 \frac{C_{13}}{E_{11}}}$. Using MADM procedure

$$L_{2x} \phi_x - \lambda_1^2 h^2 \phi_x = 0 \Rightarrow L_{2x} \phi_x = \lambda_1^2 h^2 \phi_x.$$

Pre-multiplying both sides of the equation (23) by L_{2x}^{-1} .

$$L_{2x}^{-1} L_{2x} \phi_x = L_{2x}^{-1} (\lambda_1^2 h^2 \phi_x)$$

$$\phi_x = C_5 + xC_6 + L_{2x}^{-1} \lambda_1^2 h^2 \phi_x$$

$$\phi_{x0} = \phi_{xi} = C_5 + xC_6;$$

$$\phi_{x1} = L_{2x}^{-1} (\lambda_1^2 h^2) \phi_{x0} = (\lambda_1^2 h^2) \left(C_5 \frac{x^2}{2!} + C_6 \frac{x^3}{3!} \right)$$

$$\phi_{x2} = L_{2x}^{-1} (\lambda_1^2 h^2) \phi_{x1} = (\lambda_1^2 h^2)^2 \left(C_5 \frac{x^4}{4!} + C_6 \frac{x^5}{5!} \right)$$

⋮

$$\phi_{xn} = L_{2x}^{-1} (\lambda_1^2 h^2) \phi_{x_{n-1}} = (\lambda_1^2 h^2)^n \left(C_5 \frac{x^{2n}}{2n!} + C_6 \frac{x^{2n+1}}{(2n+1)!} \right).$$

The MADM solution for equation (23) can be written as

$$\begin{aligned} \phi_x &= \sum_{i=0}^n \phi_x^i = \phi_{x0} + \phi_{x1} + \phi_{x2} + \dots + \phi_{x_{n-1}} = C_5 + xC_6 \\ &+ (\lambda_1^2 h^2) \left(C_5 \frac{x^2}{2!} + C_6 \frac{x^3}{3!} \right) + (\lambda_1^2 h^2)^2 \left(C_5 \frac{x^4}{4!} + C_6 \frac{x^5}{5!} \right) + \dots \\ &= C_5 \left(1 + (\lambda_1^2 h^2) \frac{x^2}{2!} + (\lambda_1^2 h^2)^2 \frac{x^4}{4!} + \dots + (\lambda_1^2 h^2)^n \frac{x^{2n}}{(2n)!} \right) \\ &+ C_6 \left(x + (\lambda_1^2 h^2) \frac{x^3}{3!} + (\lambda_1^2 h^2)^2 \frac{x^5}{5!} + \dots + (\lambda_1^2 h^2)^n \frac{x^{2n+1}}{(2n+1)!} \right). \end{aligned} \quad (24)$$

Hence, by using equation (22) in equation (21), u_o can be written as,

$$u_o = C_4 - \left(C_5 \left(1 + (\lambda_1^2 h^2) \frac{x^2}{2!} + (\lambda_1^2 h^2)^2 \frac{x^4}{4!} + \dots + (\lambda_1^2 h^2)^n \frac{x^{2n}}{(2n)!} \right) + C_6 \left(x + (\lambda_1^2 h^2) \frac{x^3}{3!} + (\lambda_1^2 h^2)^2 \frac{x^5}{5!} + \dots + (\lambda_1^2 h^2)^n \frac{x^{2n+1}}{(2n+1)!} \right) \right) + \frac{C_1}{E_{11}bh}. \quad (25)$$

From the third of equation (20), one can have

$$L_x \psi_x = \frac{12}{E_{11}bh^3} C_2 x + \frac{12}{E_{11}bh^3} C_3 \quad (26)$$

L_x^{-1} of above equation gives,

$$\psi_x = \frac{6}{E_{11}bh^3} C_2 x^2 + \frac{12}{E_{11}bh^3} C_3 x + C_7. \quad (27)$$

Second of equations (20) can be written as,

$$(L_x w_o + \psi_x) = \frac{C_2}{k_1^2 G_{13}bh}. \quad (28)$$

The resulting equation of L_x^{-1} of above equation can be written as,

$$w_o = \frac{C_2 x}{k_1^2 G_{13}bh} - L_x^{-1} \psi_x dx + C_8 = \frac{C_2 x}{k_1^2 G_{13}bh} - \int_0^x \psi_x dx + C_8. \quad (29)$$

By using equation (25) in equation (27), one can get

$$w_o = \frac{C_2}{E_{11}b} \left(\frac{1}{k_1^2} \frac{E_{11}}{G_{13}} \frac{x}{h} - 2 \frac{x^3}{h^3} \right) - 6 \frac{C_3}{E_{11}bh} \frac{x^2}{h^2} - C_7 h \frac{x}{h} + C_8. \quad (30)$$

The MADM solution for cracked region $[-a \leq x \leq 0]$ (beam 1) can be summarized as,

$$u_o = C_4 \left(C_5 \left(1 + (\lambda_1^2 h^2) \frac{x^2}{2!} + (\lambda_1^2 h^2)^2 \frac{x^4}{4!} + \dots + (\lambda_1^2 h^2)^n \frac{x^{2n}}{(2n)!} \right) + \frac{C_1}{E_{11}bh} + C_6 \left(x + (\lambda_1^2 h^2) \frac{x^3}{3!} + (\lambda_1^2 h^2)^2 \frac{x^5}{5!} + \dots + (\lambda_1^2 h^2)^n \frac{x^{2n+1}}{(2n+1)!} \right) \right) \quad (31a)$$

$$w_o = \frac{C_2}{E_{11}b} \left(\frac{1}{k_1^2} \frac{E_{11}}{G_{13}} \frac{x}{h} - 2 \frac{x^3}{h^3} \right) - 6 \frac{C_3}{E_{11}bh} \frac{x^2}{h^2} - C_7 h \frac{x}{h} + C_8 \quad (31b)$$

$$\phi_x = \left(C_5 \left(1 + (\lambda_1^2 h^2) \frac{x^2}{2!} + (\lambda_1^2 h^2)^2 \frac{x^4}{4!} + \dots + (\lambda_1^2 h^2)^n \frac{x^{2n}}{(2n)!} \right) + C_6 \left(x + (\lambda_1^2 h^2) \frac{x^3}{3!} + (\lambda_1^2 h^2)^2 \frac{x^5}{5!} + \dots + (\lambda_1^2 h^2)^n \frac{x^{2n+1}}{(2n+1)!} \right) \right) \quad (31c)$$

$$\psi_x = 6 \frac{C_2}{E_{11}bh} \frac{x^2}{h^2} + 12 \frac{C_3}{E_{11}bh^2} \frac{x}{h} + C_7. \quad (31d)$$

From these generalized displacement expressions, corresponding stress resultant expressions can be obtained easily by using equations (31a–31d).

Uncracked region [$0 \leq x \leq (2L - a)$] – Beams 2 and 3 (figures 2b and 3b):

In the uncracked region i.e., beams 2 and 3, $q \neq 0$. Further, axial displacement along $z = -\frac{h}{2}$ has been assumed to be zero ($z = -\frac{h}{2}$ is the bottom surface of the stress analysis mode shown in figures 2b and 3b). This can be written as,

$$u \left(z = -\frac{h}{2} \right) = 0. \quad (32)$$

By using the above equation (22), u_o can be expressed in terms of ψ_x and ϕ_x as

$$u_o = \frac{h}{2} \psi_x - \frac{h^2}{4} \phi_x \quad (33)$$

and now the surface traction ‘ q ’ is the unknown. Hence, in-plane stress resultant expressions of N_{xx} and S_{xx} will be modified, by using equation (20) as,

$$N_{xx} = \frac{\bar{E}_{11} b h^2}{2} \left(\frac{d\psi_x}{dx} - \frac{h}{3} \frac{d\phi_x}{dx} \right) = \frac{\bar{E}_{11} b h^2}{2} \left(L_x \psi_x - \frac{h}{3} L_x \phi_x \right) \quad (34a)$$

$$S_{xx} = \frac{\bar{E}_{11} b h^4}{24} \left(\frac{d\psi_x}{dx} - \frac{h}{5} \frac{d\phi_x}{dx} \right) = \frac{\bar{E}_{11} b h^4}{24} \left(L_x \psi_x - \frac{h}{5} L_x \phi_x \right). \quad (34b)$$

Further, L_x^{-1} of second of equation (15) gives,

$$Q_{xz} = \bar{C}_1. \quad (35)$$

By using the two modified in-plane stress resultant expressions N_{xx} , S_{xx} in equation (34a, b), stress resultant expression of M_{xx} and inter-laminar shear stress resultant expressions in the equation (35), first and last two equations of equation (15), one can get the following equilibrium equations in terms of generalized displacements and surface traction ‘ q ’ for uncracked region.

$$\frac{d^2 \psi_x}{dx^2} - \frac{h}{3} \frac{d^2 \phi_x}{dx^2} - \frac{2}{\bar{E}_{11} h^2} q = L_{2x} \psi_x - \frac{h}{3} L_{2x} \phi_x - \frac{2}{\bar{E}_{11} h^2} q = 0 \quad (36a)$$

$$\left(\frac{dw_o}{dx} + \psi_x \right) + \frac{f_q}{G_{13} k_1^2} q = (L_x w_o + \psi_x) + \frac{f_q}{G_{13} k_1^2} q = \frac{\bar{C}_1}{G_{13} k_1^2 b h} \quad (36b)$$

$$L_{2x} \psi_x - \frac{12}{h^2} \frac{G_{13}}{\bar{E}_{11}} \left\{ k_1^2 (L_x w_o + \psi_x) \right\} \frac{12 f_q q}{\bar{E}_{11} h^2} + \frac{6q}{\bar{E}_{11} h^2} = 0 \quad (36c)$$

$$L_{2x} \psi_x - \frac{h}{5} L_{2x} \phi_x - \frac{8k_2^2}{h} \frac{\bar{E}_{11}}{G_{13}} \phi_x + \frac{2(24f_r - 3)}{\bar{E}_{11} h^2} q = 0. \quad (36d)$$

From the first of equation (36a–36d), one can write

$$q = \frac{\bar{E}_{11} h^2}{2} \left\{ \frac{d^2 \psi_x}{dx^2} - \frac{h}{3} \frac{d^2 \phi_x}{dx^2} \right\} = \frac{\bar{E}_{11} h^2}{2} \left\{ L_{2x} \psi_x - \frac{h}{3} L_{2x} \phi_x \right\}. \quad (37)$$

Using the second of equation (36a–36d) in the third of equation (36a–36d), one can get

$$\frac{d^2\psi_x}{dx^2} - \frac{12}{h^2} \frac{\bar{C}_1}{\bar{E}_{11}bh} + \frac{6}{\bar{E}_{11}h^2} q = L_{2x}\psi_x - \frac{12}{h^2} \frac{\bar{C}_1}{\bar{E}_{11}bh} + \frac{6}{\bar{E}_{11}h^2} q = 0. \quad (38)$$

Substitution of equation (37) in equation (36) gives

$$L_{2x}\psi_x = \frac{h}{4} L_{2x}\phi_x + \frac{3}{h^2} \frac{\bar{C}_1}{\bar{E}_{11}bh} \quad (39)$$

L_x^{-1} of equation (39) twice gives,

$$\psi_x = \frac{h}{4} \phi_x + \frac{3}{2} \frac{\bar{C}_1 x^2}{\bar{E}_{11}bh^3} + \bar{C}_2 x + \bar{C}_3. \quad (40)$$

Expression for q can be rewritten, by using equation (39) in equation (37), as

$$q = -\frac{\bar{E}_{11}h^3}{24} L_{2x}\phi_x + \frac{3}{2} \frac{\bar{C}_1}{bh}. \quad (41)$$

From the second of equation (36a–36d), one can have

$$(L_x w_o + \psi_x) = -\frac{f_q}{G_{13}k_1^2} q + \frac{\bar{C}_1}{G_{13}k_1^2 bh}. \quad (42)$$

By multiplying L_x^{-1} the above equation (27) and rearranging, one can get,

$$\begin{aligned} w_o &= -L_x^{-1}\psi_x dx - \frac{f_q}{G_{13}k_1^2} L_x^{-1} q dx + \frac{\bar{C}_1 x}{G_{13}k_1^2 bh} + \bar{C}_4 \\ &= -\int_0^x \psi_x dx - \frac{f_q}{G_{13}k_1^2} \int_0^x q dx + \frac{\bar{C}_1 x}{G_{13}k_1^2 bh} + \bar{C}_4. \end{aligned}$$

Substitution of equations (24) and (26) in the fourth of equation (36) results in the following equation:

$$L_{2x}\phi_x - \frac{\lambda_2^2}{h^2} \phi_x = \frac{\bar{S}_1}{\bar{E}_{11}bh^2} \bar{C}_1, \quad (43)$$

where $\lambda_2 = \sqrt{\frac{80k_2^2}{(3-20f_r)} \frac{G_{13}}{\bar{E}_{11}}}$, $\bar{S}_1 = -\frac{30(24f_r-2)}{(3-20f_r)}$.

The MADM solution for equation (43) can be written as

$$L_{2x}\phi_x = \frac{\lambda_2^2}{h^2} \phi_x + \frac{\bar{S}_1}{\bar{E}_{11}bh^2} \bar{C}_1.$$

Pre-multiplying both sides of the equation (43) by L_{2x}^{-1} .

$$\begin{aligned}
L_{2x}^{-1}L_{2x}\phi_x &= L_{2x}^{-1}\left(\frac{\lambda_2^2}{h^2}\phi_x + \frac{\bar{S}_1}{E_{11}bh^2}\bar{C}_1\right) \\
\phi_x &= \bar{C}_5 + x\bar{C}_6 + L_{2x}^{-1}\left(\frac{\lambda_2^2}{h^2}\phi_x + \frac{\bar{S}_1}{E_{11}bh^2}\bar{C}_1\right) \\
\phi_{xi} &= \bar{C}_5 + x\bar{C}_6 + L_{2x}^{-1}\left(\frac{\bar{S}_1}{E_{11}bh^2}\bar{C}_1\right) = \bar{C}_5 + x\bar{C}_6 + \left(\frac{\bar{S}_1}{E_{11}bh^2}\bar{C}_1\right)\frac{x^2}{2!} \\
\phi_{x0} &= \bar{C}_5 + x\bar{C}_6 \\
\phi_{x1} &= L_{2x}^{-1}\left(\frac{\bar{S}_1}{E_{11}bh^2}\bar{C}_1\right) + L_{2x}^{-1}\left(\frac{\lambda_2^2}{h^2}\phi_{x0}\right) \\
&= \frac{\bar{S}_1}{E_{11}bh^2}\bar{C}_1\frac{x^2}{2!} + \left(\frac{\lambda_2^2}{h^2}\right)\left(\bar{C}_5\frac{x^2}{2!} + \bar{C}_6\frac{x^3}{3!}\right) \\
\phi_{x2} &= L_{2x}^{-1}\left(\frac{\lambda_2^2}{h^2}\phi_{x1}\right) = \left(\frac{\lambda_2^2}{h^2}\right)^2\left(\bar{C}_5\frac{x^4}{4!} + \bar{C}_6\frac{x^5}{5!}\right) + \left(\frac{\lambda_2^2}{h^2}\right)\left(\frac{\bar{S}_1}{E_{11}bh^2}\bar{C}_1\frac{x^6}{6!}\right) \\
&\vdots \\
\phi_{xn} &= L_{2x}^{-1}\left(\frac{\lambda_2^2}{h^2}\phi_{xn-1}\right) = \left(\frac{\lambda_2^2}{h^2}\right)^n\left(\bar{C}_5\frac{x^{2n}}{2n!} + \bar{C}_6\frac{x^{2n+1}}{2n+1!}\right) + \left(\frac{\lambda_2^2}{h^2}\right)^{n-1}\left(\frac{\bar{S}_1}{E_{11}bh^2}\bar{C}_1\frac{x^{2(n+1)}}{2(n+1)!}\right).
\end{aligned}$$

Hence, the MADM solution for equation (43) can be written as

$$\begin{aligned}
\phi_x &= \bar{C}_5\left(1 + \left(\frac{\lambda_2^2}{h^2}\right)\frac{x^2}{2!} + \left(\frac{\lambda_2^2}{h^2}\right)^2\frac{x^4}{4!} + \left(\frac{\lambda_2^2}{h^2}\right)^3\frac{x^6}{6!} + \dots + \left(\frac{\lambda_2^2}{h^2}\right)^n\frac{x^{2n}}{(2n)!}\right) \\
&+ \bar{C}_6\left(x + \left(\frac{\lambda_2^2}{h^2}\right)\frac{x^3}{3!} + \left(\frac{\lambda_2^2}{h^2}\right)^2\frac{x^5}{5!} + \left(\frac{\lambda_2^2}{h^2}\right)^3\frac{x^7}{7!} + \dots + \left(\frac{\lambda_2^2}{h^2}\right)^n\frac{x^{2n+1}}{(2n+1)!}\right) \\
&+ \frac{\bar{S}_1}{E_{11}bh^2}\bar{C}_1\left(\frac{x^2}{2!} + \left(\frac{\lambda_2^2}{h^2}\right)\frac{x^4}{4!} + \left(\frac{\lambda_2^2}{h^2}\right)^2\frac{x^6}{6!} + \left(\frac{\lambda_2^2}{h^2}\right)^3\frac{x^8}{8!}\right. \\
&\quad \left.+ \dots + \left(\frac{\lambda_2^2}{h^2}\right)^{n-1}\frac{x^{2(n+1)}}{(2(n+1))!}\right). \tag{44}
\end{aligned}$$

Substitution of equation (45) in the equation (40) gives

$$\psi_x = \frac{\bar{C}_1}{E_{11}bh}\left(\frac{3x^2}{2h^2}\right) + \bar{C}_2x + \bar{C}_3 + \phi_x. \tag{45}$$

Using equation (45) in the equation (41), one can get,

$$\begin{aligned}
 q = & -\frac{\bar{E}_{11}h^3}{24}L_{2x} \left(\bar{C}_5 \left(1 + \left(\frac{\lambda_2^2}{h^2}\right) \frac{x^2}{2!} + \left(\frac{\lambda_2^2}{h^2}\right)^2 \frac{x^4}{4!} + \left(\frac{\lambda_2^2}{h^2}\right)^3 \frac{x^6}{6!} + \dots + \left(\frac{\lambda_2^2}{h^2}\right)^n \frac{x^{2n}}{(2n)!} \right) \right. \\
 & + \bar{C}_6 \left(x + \left(\frac{\lambda_2^2}{h^2}\right) \frac{x^3}{3!} + \left(\frac{\lambda_2^2}{h^2}\right)^2 \frac{x^5}{5!} + \left(\frac{\lambda_2^2}{h^2}\right)^3 \frac{x^7}{7!} + \dots + \left(\frac{\lambda_2^2}{h^2}\right)^n \frac{x^{2n+1}}{(2n+1)!} \right) + \frac{3}{2} \frac{\bar{C}_1}{bh} \\
 & + \frac{\bar{S}_1}{\bar{E}_{11}bh^2} \bar{C}_1 \left(\frac{x^2}{2!} + \left(\frac{\lambda_2^2}{h^2}\right) \frac{x^4}{4!} + \left(\frac{\lambda_2^2}{h^2}\right)^2 \frac{x^6}{6!} + \left(\frac{\lambda_2^2}{h^2}\right)^3 \frac{x^8}{8!} \right. \\
 & \left. + \dots + \left(\frac{\lambda_2^2}{h^2}\right)^{n-1} \frac{x^{2(n+1)}}{(2(n+1))!} \right).
 \end{aligned} \tag{46}$$

Substitution of equation (45), (46) in the equation (43) gives,

$$\begin{aligned}
 w_o = & \frac{\bar{C}_1}{\bar{E}_{11}b} \left\{ \bar{S}_2 \frac{x}{h} - \frac{1}{2} \frac{x^3}{h^3} \right\} - \bar{C}_2 \frac{x^2}{2} - \bar{C}_3 x + \bar{C}_4 \\
 & + h^2 \bar{S}_3 \bar{C}_5 \left(1 + \left(\frac{\lambda_2^2}{h^2}\right) \frac{x^2}{2!} + \left(\frac{\lambda_2^2}{h^2}\right)^2 \frac{x^4}{4!} + \left(\frac{\lambda_2^2}{h^2}\right)^3 \frac{x^6}{6!} + \dots + \left(\frac{\lambda_2^2}{h^2}\right)^n \frac{x^{2n}}{(2n)!} \right) \\
 & - h^2 \bar{S}_3 \bar{C}_6 \left(x + \left(\frac{\lambda_2^2}{h^2}\right) \frac{x^3}{3!} + \left(\frac{\lambda_2^2}{h^2}\right)^2 \frac{x^5}{5!} + \left(\frac{\lambda_2^2}{h^2}\right)^3 \frac{x^7}{7!} + \dots + \left(\frac{\lambda_2^2}{h^2}\right)^n \frac{x^{2n+1}}{(2n+1)!} \right) \\
 & + h^2 \bar{S}_3 \frac{\bar{S}_1}{\bar{E}_{11}bh^2} \bar{C}_1 \left(\frac{x^2}{2!} + \left(\frac{\lambda_2^2}{h^2}\right) \frac{x^4}{4!} + \left(\frac{\lambda_2^2}{h^2}\right)^2 \frac{x^6}{6!} + \left(\frac{\lambda_2^2}{h^2}\right)^3 \frac{x^8}{8!} \right. \\
 & \left. + \dots + \left(\frac{\lambda_2^2}{h^2}\right)^{n-1} \frac{x^{2(n+1)}}{(2(n+1))!} \right),
 \end{aligned} \tag{47}$$

$$\text{where } \bar{S}_2 = \left(1 - \frac{3}{2} f_q \right) \frac{\bar{E}_{11}}{G_{13}} \frac{1}{k_1^2} + \frac{\bar{S}_1}{4\lambda_2^2},$$

$$\bar{S}_3 = \lambda_2^2 \left(\frac{f_q}{24k_1^2} \frac{\bar{E}_{11}}{G_{13}} - \frac{1}{4\lambda_2^2} \right).$$

For the uncracked region $[0 \leq x \leq (L - a)]$ (beam 2), solution can be summarized as

$$\begin{aligned}
 w_o = & \frac{\bar{C}_1}{\bar{E}_{11}b} \left\{ \bar{S}_2 \frac{x}{h} - \frac{1}{2} \frac{x^3}{h^3} \right\} - \bar{C}_2 \frac{x^2}{2} - \bar{C}_3 x + \bar{C}_4 + \bar{C}_5 h^2 \bar{S}_3 \\
 & \left(1 + \left(\frac{\lambda_2^2}{h^2}\right) \frac{x^2}{2!} + \left(\frac{\lambda_2^2}{h^2}\right)^2 \frac{x^4}{4!} + \left(\frac{\lambda_2^2}{h^2}\right)^3 \frac{x^6}{6!} + \dots + \left(\frac{\lambda_2^2}{h^2}\right)^n \frac{x^{2n}}{(2n)!} \right) \\
 & - \bar{C}_6 h^2 \bar{S}_3 \left(x + \left(\frac{\lambda_2^2}{h^2}\right) \frac{x^3}{3!} + \left(\frac{\lambda_2^2}{h^2}\right)^2 \frac{x^5}{5!} + \dots + \left(\frac{\lambda_2^2}{h^2}\right)^n \frac{x^{2n+1}}{(2n+1)!} \right)
 \end{aligned} \tag{48a}$$

$$\begin{aligned}
\psi_x = & \frac{\bar{C}_1}{\bar{E}_{11}bh} \left(\frac{3x^2}{2h^2} - \frac{\bar{S}_1}{4\lambda_2^2} \right) + \bar{C}_2x + \bar{C}_3 + \bar{C}_5 \frac{h}{4} \\
& \left(1 + \left(\frac{\lambda_2^2}{h^2} \right) \frac{x^2}{2!} + \left(\frac{\lambda_2^2}{h^2} \right)^2 \frac{x^4}{4!} + \left(\frac{\lambda_2^2}{h^2} \right)^3 \frac{x^6}{6!} + \dots + \left(\frac{\lambda_2^2}{h^2} \right)^n \frac{x^{2n}}{(2n)!} \right) \\
& + \bar{C}_6 \frac{h}{4} \left(x + \left(\frac{\lambda_2^2}{h^2} \right) \frac{x^3}{3!} + \left(\frac{\lambda_2^2}{h^2} \right)^2 \frac{x^5}{5!} + \dots + \left(\frac{\lambda_2^2}{h^2} \right)^n \frac{x^{2n+1}}{(2n+1)!} \right) \quad (48b)
\end{aligned}$$

$$\begin{aligned}
\phi_x = & \frac{\bar{S}_1}{\bar{E}_{11}bh^2} \bar{C}_1 \left(\frac{x^2}{2!} + \left(\frac{\lambda_2^2}{h^2} \right) \frac{x^4}{4!} + \left(\frac{\lambda_2^2}{h^2} \right)^2 \frac{x^6}{6!} + \dots + \left(\frac{\lambda_2^2}{h^2} \right)^{n-1} \frac{x^{2(n+1)}}{(2(n+1))!} \right) \\
& + \bar{C}_5 \left(1 + \left(\frac{\lambda_2^2}{h^2} \right) \frac{x^2}{2!} + \left(\frac{\lambda_2^2}{h^2} \right)^2 \frac{x^4}{4!} + \left(\frac{\lambda_2^2}{h^2} \right)^3 \frac{x^6}{6!} + \dots + \left(\frac{\lambda_2^2}{h^2} \right)^n \frac{x^{2n}}{(2n)!} \right) \\
& + \bar{C}_6 \left(x + \left(\frac{\lambda_2^2}{h^2} \right) \frac{x^3}{3!} + \left(\frac{\lambda_2^2}{h^2} \right)^2 \frac{x^5}{5!} + \dots + \left(\frac{\lambda_2^2}{h^2} \right)^n \frac{x^{2n+1}}{(2n+1)!} \right) \quad (48c)
\end{aligned}$$

$$\begin{aligned}
q = & \frac{3\bar{C}_1}{2bh} - \frac{\bar{E}_{11}\lambda_2^2h}{24} \left(\bar{C}_5 \left(1 + \left(\frac{\lambda_2^2}{h^2} \right) \frac{x^2}{2!} + \left(\frac{\lambda_2^2}{h^2} \right)^2 \frac{x^4}{4!} + \dots + \left(\frac{\lambda_2^2}{h^2} \right)^n \frac{x^{2n}}{(2n)!} \right) \right. \\
& \left. + \bar{C}_6 \left(x + \left(\frac{\lambda_2^2}{h^2} \right) \frac{x^3}{3!} + \left(\frac{\lambda_2^2}{h^2} \right)^2 \frac{x^5}{5!} + \dots + \left(\frac{\lambda_2^2}{h^2} \right)^n \frac{x^{2n+1}}{(2n+1)!} \right) \right). \quad (48d)
\end{aligned}$$

Similar solution can be obtained for uncracked region $[(L-a) \leq x \leq (2L-a)]$ (beam 3) and the solution can be written, with different integration constants as

$$\begin{aligned}
w_o = & \frac{\tilde{C}_1}{\tilde{E}_{11}b} \left\{ \tilde{S}_2 \frac{x}{h} - \frac{1}{2} \frac{x^3}{h^3} \right\} - \tilde{C}_2 \frac{x^2}{2} - \tilde{C}_3x + \tilde{C}_4 + \tilde{C}_5h^2\tilde{S}_3 \\
& \left(1 + \left(\frac{\lambda_2^2}{h^2} \right) \frac{x^2}{2!} + \left(\frac{\lambda_2^2}{h^2} \right)^2 \frac{x^4}{4!} + \left(\frac{\lambda_2^2}{h^2} \right)^3 \frac{x^6}{6!} + \dots + \left(\frac{\lambda_2^2}{h^2} \right)^n \frac{x^{2n}}{(2n)!} \right) \\
& - \tilde{C}_6h^2\tilde{S}_3 \left(x + \left(\frac{\lambda_2^2}{h^2} \right) \frac{x^3}{3!} + \left(\frac{\lambda_2^2}{h^2} \right)^2 \frac{x^5}{5!} + \dots + \left(\frac{\lambda_2^2}{h^2} \right)^n \frac{x^{2n+1}}{(2n+1)!} \right) \quad (49a)
\end{aligned}$$

$$\begin{aligned}
\psi_x = & \frac{\tilde{C}_1}{\tilde{E}_{11}bh} \left\{ \frac{3x^2}{2h^2} - \frac{\tilde{S}_1}{4\lambda_2^2} \right\} + \tilde{C}_2x + \tilde{C}_3 + \tilde{C}_5 \frac{h}{4} \\
& \left(1 + \left(\frac{\lambda_2^2}{h^2} \right) \frac{x^2}{2!} + \left(\frac{\lambda_2^2}{h^2} \right)^2 \frac{x^4}{4!} + \left(\frac{\lambda_2^2}{h^2} \right)^3 \frac{x^6}{6!} + \dots + \left(\frac{\lambda_2^2}{h^2} \right)^n \frac{x^{2n}}{(2n)!} \right) + \\
& \tilde{C}_6 \frac{h}{4} \left(x + \left(\frac{\lambda_2^2}{h^2} \right) \frac{x^3}{3!} + \left(\frac{\lambda_2^2}{h^2} \right)^2 \frac{x^5}{5!} + \dots + \left(\frac{\lambda_2^2}{h^2} \right)^n \frac{x^{2n+1}}{(2n+1)!} \right) \quad (49b)
\end{aligned}$$

$$\begin{aligned}
\phi_x = & \frac{\tilde{S}_1}{\tilde{E}_{11}bh^2} \tilde{C}_1 \left(\frac{x^2}{2!} + \left(\frac{\lambda_2^2}{h^2}\right)^2 \frac{x^4}{4!} + \left(\frac{\lambda_2^2}{h^2}\right)^2 \frac{x^6}{6!} + \dots + \left(\frac{\lambda_2^2}{h^2}\right)^2 \frac{x^{2n+1}}{(2(n+1))!} \right) \\
& + \tilde{C}_5 \left(1 + \left(\frac{\lambda_2^2}{h^2}\right) \frac{x^2}{2!} + \left(\frac{\lambda_2^2}{h^2}\right)^2 \frac{x^4}{4!} + \left(\frac{\lambda_2^2}{h^2}\right)^3 \frac{x^6}{6!} + \dots + \left(\frac{\lambda_2^2}{h^2}\right)^n \frac{x^{2n}}{(2n)!} \right) \\
& + \tilde{C}_6 \left(x + \left(\frac{\lambda_2^2}{h^2}\right) \frac{x^3}{3!} + \left(\frac{\lambda_2^2}{h^2}\right)^2 \frac{x^5}{5!} + \dots + \left(\frac{\lambda_2^2}{h^2}\right)^n \frac{x^{2n+1}}{(2n+1)!} \right) \quad (49c)
\end{aligned}$$

$$\begin{aligned}
q = & \frac{3\tilde{C}_1}{2bh} - \frac{\tilde{E}_{11}\lambda_2^2h}{24} \left(\tilde{C}_5 \left(1 + \left(\frac{\lambda_2^2}{h^2}\right) \frac{x^2}{2!} + \left(\frac{\lambda_2^2}{h^2}\right)^2 \frac{x^4}{4!} + \dots + \left(\frac{\lambda_2^2}{h^2}\right)^n \frac{x^{2n}}{(2n)!} \right) \right. \\
& \left. + \tilde{C}_6 \left(x + \left(\frac{\lambda_2^2}{h^2}\right) \frac{x^3}{3!} + \left(\frac{\lambda_2^2}{h^2}\right)^2 \frac{x^5}{5!} + \dots + \left(\frac{\lambda_2^2}{h^2}\right)^n \frac{x^{2n+1}}{(2n+1)!} \right) \right). \quad (49d)
\end{aligned}$$

From these generalized displacement expressions, corresponding stress resultant expressions can be obtained easily by using equations (34) and (16). According to SOBT, the unknown constants (for ENF and ENC specimens) will have to be determined from boundary and matching conditions given in the sections 2.2 and 2.3 for ENF and ENC specimens, respectively. These simultaneous equations have been solved using MADM for the unknown constants. Once these constants are determined, deflection under the load (at $x = (L - a)$ for ENF specimen and at $x = -a$ for ENC specimen) has been determined using equations (32) and (18) for ENF and ENC specimens, respectively. From the deflection thus obtained, compliance and then SERR have been obtained using the procedures presented in section 3.

3. Determination of compliance and strain energy release rate

The compliance 'C' can be obtained from the following relation $C = \frac{\delta}{P}$ in which 'δ' is the deflection under the load P. In the case of ENF specimen, 'δ' is obtained from $\delta = w_o^{(2)}$ [at $x = (L - a)$] or $w_o^{(3)}$ [at $x = (L - a)$]. Similarly, in the case of ENC specimen, 'δ' is obtained from $\delta = w_o^{(1)}$ [at $x = -a$]. The SERR and the compliance are related by the following formula $G_{II} = \frac{P^2}{2b} \frac{dC}{da}$. In the present paper, $\frac{dC}{da}$ has not been determined explicitly as it is tedious. And hence, the derivative $\frac{dC}{da}$ has been evaluated using MADM spatial discretization. Hence, one can obtain SERR from the following expression $G_{II} = \frac{P^2}{2b} \left(\frac{C_n - C_{n-1}}{\Delta} \right) = \frac{P^2}{2b} \left(\frac{C_{n+1} - C_n}{\Delta} \right)$ (figure 4).

4. Numerical examples

In this section, first to validate present formulation, the compliance and the SERR, obtained from the analysis of unidirectional ENF and ENC specimens and using present formulation considering various theories FOBT, SOBT^{1,2} can be compared with the available results mentioned in Carlsson *et al* (1986a), Whitney (1990), Whitney *et al* (1987), Salpekar *et al* (1988), Sela *et al* (1989), Pavankumar & Raghu Prasad (2003), Chatterjee (1991), Wang & Williams (1992) in the literature. Further, parametric study can be carried out to study the influence of crack length(a),

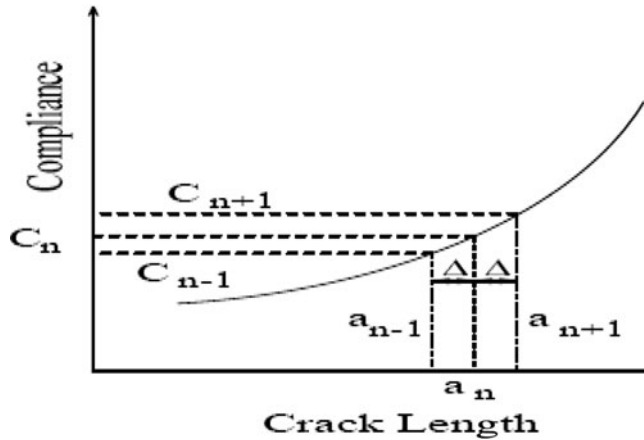


Figure 4. Numerical and compliance approach.

span to depth ($\frac{L}{h}$) ratio and ratio of Young's modulus to shear modulus (shear deformation) ($\frac{E_{11}}{G_{13}}$) on the compliance, SERR and shear stress distribution ahead of the crack tip. Also, comparative study can be made among the various laminated composite beam theories considered to bring out the importance of shear deformation theories i.e., FOBT and SOBT, clearly and particularly in the case of SERR.

4.1 Geometrical, material properties and load for ENC and ENF problems

Table 3 gives the material and geometrical properties as given by Sela *et al* (1989).

MADM Results: The results obtained from the present work have been compared with the experimental results in table 4. Comparative study has been made with the results of Wang & Williams (1992) and for this purpose material and geometrical properties and load have been

Table 3. Material properties and geometrical properties from Sela *et al* (1989).

Material Properties: [†]
$E_{11} = 1.4 \times 10^5$, $E_{22} = 1 \times 10^4$
$G_{13} = 6 \times 10^3$
$\nu_{13} = 0.34$
Geometrical Properties:
$L =$ half span = 50.8
$b =$ width 25.4
$h =$ half thickness = 1.524
No. of laminae = 24, N = Lamina thickness = 0.127
$a =$ crack length = 25.4
$P_c =$ Load = 672.9

[†] All the dimensions are mentioned in N, mm by default

Table 4. Comparison of compliance and SERR from MADM with experimental results of Sela *et al* (1989), and analytical from Raghu Prasad & Pavankumar (2008).

Mode	Compliance $\frac{\text{mm}}{\text{N}}$		SERR G_{IIC} $\frac{\text{J}}{\text{m}^2}$	
Sela <i>et al</i> (1989)	0.0032		527	
	Raghu Prasad & Pavankumar (2008)		MADM	
FOBT	0.0031	494.7750	0.00312	516.4701
SOBT ¹	0.00315	494.775	0.00319	513.461
SOBT ²	0.00316	494.775	0.00321	517.150
SOBT ^E	0.00318	495.468	0.00321	528.375

given in the tables where the results have been presented in tables 4, 5, 6, 7, 8 and 9. Compliance and SERR values obtained from the present work have been compared with the experimentally obtained compliance and SERR values of Sela *et al* (1989). These comparisons have been presented in table 5. By closely examining table 5, it can be concluded that, among FOBT and SOBT^{1,2,E} beam theories considered, compliance and SERR values obtained from SOBT^E are in good agreement with the experimental and analytical obtained compliance and SERR values. The present work results have also been compared with those of Wang & Williams (1992) for both ENF and ENC (also called as End Loaded Split (ELS) specimen) specimens and are presented in the tables 5, 6, 7, 8 and 9. Once again it can be observed that results from SOBT are in better agreement with those from experimental and FEM values of Wang & Williams (1992) when compared to those of SOBT and also compared analytical results too. Tables 7 and 9 also reveal the fact as to how important is the SOBT deformation in calculating the SERR accurately. Having established the point that one can use plane stress type of analysis, in the further presentation plane stress type analysis results have been considered and also it is due to the reason that the width of the specimen is larger than its thickness. Further, parametric study can be carried out only for unidirectional ENF specimen because the conclusions that could be drawn

Table 5. Comparison of compliance and SERR from MADM with Wang & Williams (1992) & Raghu Prasad & Pavankumar (2008) results for ENF specimen.

Material ¹ : $S_{11} = S_{22} = 7.69 \times 10^{-6}$, $S_{66} = 2.5 \times 10^{-4}$, $\nu_{12} = 0.3$.						
Geometry: $L = 50$, $b = 1.0$, $h = 1.5$, $P = 1.0$.						
a	Wang & Williams (1992)		Raghu Prasad & Pavankumar (2008)	MADM		
	Compliance ($\frac{\text{mm}}{\text{N}}$)			SOBT ¹	SOBT ²	SOBT ^E
10	0.075		0.075	0.074	0.075	0.075
20	0.082		0.082	0.081	0.082	0.081
30	0.100		0.099	0.097	0.100	0.098
40	0.134		0.133	0.128	0.133	0.131
SERR $\times 10^3$ ($\frac{\text{N-mm}}{\text{mm}^2}$)						
	FEM	Semi-empirical				
10	0.163	0.160		0.146	0.157	0.155
20	0.597	0.573		0.546	0.570	0.565
30	1.293	1.243		1.203	1.238	1.231
40	2.230	2.170		2.116	2.161	2.152

Table 6. Comparison of compliance and SERR from MADM with Wang & Williams (1992) and Raghu Prasad & Pavankumar (2008) results for ENF specimen.

Material ² : $S_{11} = 6.8 \times 10^{-6}$, $S_{22} = 128 \times 10^{-3}$, $S_{66} = 362 \times 10^{-3}$. Geometry ² : Geometry, Poisson's ratio and Load same as material ¹ .							
a	Wang & Williams (1992)		Raghu Prasad & Pavankumar (2008)		MADM		
					SOBT ¹	SOBT ²	SOBT ^E
Compliance ($\frac{\text{mm}}{\text{N}}$)							
10	0.0676		0.0669		0.0676	0.0681	0.0689
20	0.0738		0.0723		0.0735	0.0744	0.0752
30	0.0896		0.0867		0.0888	0.0903	0.0914
40	0.1197		0.1219		0.1181	0.1205	0.1147
SERR $\times 10^3$ ($\frac{\text{N-mm}}{\text{mm}^2}$)							
	FEM	Semi-empirical					
10	0.151	0.151		0.144	0.134	0.147	0.116
20	0.551	0.526		0.513	0.492	0.518	0.456
30	1.182	1.128		1.108	1.077	1.116	1.022
40	2.025	1.957		1.927	1.887	1.936	1.816

for unidirectional ENF specimen could be valid for unidirectional ENC specimen also. Values of compliance, SERR and shear stress distribution ahead of the crack tip based on SOBT only could be presented in the plots in tables for the SOBT category. And, it should be noted that FOBT and SOBT¹ can show the same trend as SOBT^{2,E}, respectively with minor differences in the magnitudes of the results.

4.1a *Influence of crack length ($\frac{a}{L}$) ratio:* The above tables show the variation of compliance and SERR, obtained from shear deformation theories. From these tables, it is observed that compliance and SERR increase as crack length increases. Compliance and SERR given by shear deformation theories are more than those from FOBT which reflects the importance of first

Table 7. Comparison of SERR from MADM with Wang & Williams (1992) and Raghu Prasad & Pavankumar (2008) results for ENF specimen.

Material ³ : Material properties, Poisson's ratio and load same as material ² . Geometry ³ : $a = 20$, $L = 50$, $b = 1.0$, $h = 1.5$.							
S_{66}	Wang & Williams (1992)		Raghu Prasad & Pavankumar (2008)		MADM		
					SOBT ¹	SOBT ²	SOBT ^E
SERR $\times 10^3$ ($\frac{\text{N-mm}}{\text{mm}^2}$)							
	FEM	Semi-empirical					
181×10^{-6}	0.527	0.508		0.455	0.480	0.499	0.495
362×10^{-6}	0.551	0.526		0.456	0.492	0.518	0.513
724×10^{-6}	0.587	0.552		0.458	0.510	0.546	0.538
145×10^{-5}	0.637	0.592		0.462	0.536	0.587	0.575
500×10^{-5}	0.783	0.719		0.485	0.614	0.692	0.682

Table 8. Comparison of compliance and SERR from MADM with Wang & Williams (1992) and Raghu Prasad & Pavankumar (2008) results for ENC specimen.

Material ⁴ : Material property, Poisson's ratio and Load same as material ² .						
Geometry ⁴ : $L = 60, b = 4.1, h = 1.0$.						
a	Wang & Williams (1992)	Raghu Prasad & Pavankumar (2008)	MADM			
			SOBT ¹	SOBT ²	SOBT ^E	
Compliance ($\frac{\text{mm}}{\text{N}}$)						
2	0.9215	0.7459	0.7480	0.7491	0.7476	
5	0.9234	0.7472	0.7496	0.7510	0.7494	
10	0.9347	0.7563	0.7599	0.7621	0.7604	
20	1.0160	0.8280	0.8365	0.8423	0.8397	
30	1.2260	1.0220	1.0388	1.0505	1.0466	
40	1.6280	1.3997	1.4279	1.4479	1.4422	
50	2.2820	2.0222	2.0650	2.0957	2.0878	
55	2.7220	2.4443	2.4955	2.5316	2.5229	
SERR $\times 10^2$ ($\frac{\text{N-mm}}{\text{mm}^2}$)						
	FEM J-Integral	Semi-empirical				
2	0.0134	0.014	0.0075	0.0107	0.0127	0.0123
5	0.054	0.056	0.040	0.048	0.054	0.052
10	0.183	0.186	0.154	0.171	0.183	0.180
20	0.672	0.677	0.613	0.646	0.670	0.665
30	1.466	1.474	1.378	1.427	1.463	1.455
40	2.567	2.577	2.449	2.515	2.562	2.552
50	3.972	3.987	3.826	3.908	3.966	3.954
55	4.752	4.806	4.630	4.708	4.739	4.747

order shear deformation. Also tables show that the influence of crack length on the normalized compliance and SERR values obtained from the shear deformation theories. From table, it can be observed that normalized compliance values from SOBT¹ show descending trend which

Table 9. Comparison of SERR from MADM with Wang & Williams (1992) and Raghu Prasad & Pavankumar (2008) results for ENC specimen.

Material ⁵ : Material property, Poisson's ratio and load same as material ² .						
Geometry ⁵ : Geometry property same as geometry ² .						
S_{66}	Wang & Williams (1992)	Raghu Prasad & Pavankumar (2008)	MADM			
			SOBT ¹	SOBT ²	SOBT ^E	
SERR $\times 10^2$ ($\frac{\text{N-mm}}{\text{mm}^2}$)						
	FEM	Semi-empirical				
200×10^{-7}	0.647	0.634	0.612	0.620	0.625	0.624
181×10^{-6}	0.661	0.662	0.6127	0.636	0.653	0.649
362×10^{-6}	0.672	0.677	0.6134	0.646	0.670	0.665
724×10^{-6}	0.697	0.700	0.615	0.661	0.695	0.687
145×10^{-5}	0.727	0.735	0.617	0.683	0.730	0.720
500×10^{-5}	0.877	0.842	0.631	0.754	0.839	0.820

means that the theory $SOBT^E$ tend towards FOBT as crack length increases. Normalized compliances from SOBT show ascending trend up to certain crack lengths and then afterwards show a descending trend based on the material property under consideration. In other words, based on the material properties, the difference between the compliance values from FOBT and SOBT theories increases up to a certain crack length and then afterwards decreases. The SERR values of $SOBT^E$ approach those of FOBT as the crack length increases which can be seen clearly. The shear stress distribution ahead of the crack tip obtained from FOBT and SOBT beam theories for any $\frac{a}{L}$ (given crack length) is the shear stress uniform throughout i.e., constant. It could be seen that shear stress given by FOBT and $SOBT^{1,2}$ is constant ahead of the crack tip. It implies that FOBT does not affect the shear stress distribution in the case of $SOBT^E$ (i.e., independent of the shear deformation). Peak shear stress values of $SOBT^E$ are slightly greater than those of FOBT.

5. Results and discussion

In this section, mode II fracture of one-dimensional layered laminate is analysed by MADM. The basic governing differential equations of equilibrium and matching conditions are borrowed from earlier work by Raghu Prasad & Pavankumar (2008), Pavankumar & Raghu Prasad (2003, 2008, 2009), Pavankumar (2004). While familiar methods already known to solve governing differential equations were employed by the earlier researchers as mentioned above. MADM is employed here. It is very interesting to see that MADM gives results which are closer to the experimental than the earlier methods (classical) as mentioned in Salpekar *et al* (1988), Sela *et al* (1989), Gillespie *et al* (1986), Wang & Williams (1992), Chatterjee (1991), Whitney *et al* (1987), Whitney (1990), Corleto & Hogan (1995), Carlsson *et al* (1986a).

It is evident that the results from MADM are much closer to the experimental than the results due to analytical methods employed by earlier researchers mentioned above. The reason is quite obvious. In the earlier works mentioned above, after obtaining the compliance C , the derivative $\frac{dC}{da}$ was obtained by finite difference scheme, while in the present work it was not required. Not only compliance values C were obtained by MADM, even $\frac{dC}{da}$ values were obtained by MADM, which means errors arising due to the use of numerical schemes such as finite difference do not appear here. Here MADM values are represented in FOBT, $SOBT^{1,2}$ and analytical values are represented in TOBT. Table 10 shows the values of compliance (C) and SERR G_{II}

Table 10. Difference in values between bench mark and MADM results.

Table No:	Bench Mark	Parameter	Results				Analytical BKR <i>et al</i>
			MADM				
			FOBT ^E	SOBT ¹	SOBT ²	SOBT ^E	
4	Expt.	Compl.	0.0008	0.00001	-0.00001	-0.00001	0.00002
5	Expt.	Compl.		0.001	0	0	0
	FEM	SERR		0.0170	0.0060	0.0080	0.0330
6	Expt.	Compl.		0	-0.0005	-0.0013	0.0007
	FEM	SERR		0.0170	0.0040	0.0350	0.0070
7	FEM	SERR		0.047	0.028	0.032	0.072
8	Expt.	Compl.		0.1756	0.1735	0.1739	0.1743
	FEM	SERR		0.0027	0.0007	0.0011	0.0059
9	FEM	SERR		0.027	0.022	0.023	0.035

as obtained by Wang & Williams (1992), Gillespie *et al* (1986), Carlsson *et al* (1986a), Whitney *et al* (1987), Whitney (1990) from experiments by Raghu Prasad & Pavankumar (2008), Pavankumar & Raghu Prasad (2003, 2008, 2009), Pavankumar (2004) by classical methods and by MADM in the current work. It is encouraging to note that MADM results are close and agree with the exact as well as analytical in trend.

The results show that the compliance (C) and SERR G_{II} values are obtained by MADM are closer to the experimental ones by Salpekar *et al* (1988) than those of Pavankumar and Raghu Prasad as mentioned above, while the latter have adopted third order shear deformation theory. In other words, using MADM it is possible to get results closer to the experimental one without resorting to the higher order theories.

6. Conclusions

Mathematical models for the stress analysis of unidirectional ENF and ENC specimens using FOBT and SOBT have been developed and presented for inter-laminar fracture toughness of unidirectional composites in mode II. For this purpose, the available stress analysis models (Whitney 1990) for ENF and ENC specimens have been considered. These models consider only upper halves of ENF and ENC specimens because the delamination is at mid-plane and lamination scheme is symmetric about the mid-plane of ENF and ENC specimens. In the present paper, appropriate matching conditions, in terms of generalized displacements and stress resultants, have been applied at the crack tip by enforcing the displacement continuity at the crack tip in conjunction with variational equation. SERR has been calculated using compliance approach. The compliance and SERR obtained from the present formulations have been compared with the existing experimental, analytical and finite element results, it is observed that MADM is in good agreement with the existing results. Also, parametric study has been carried out to study the influence of crack length, ratio of Young's modulus to shear modulus and span to depth ratio of the specimen on the compliance and SERR. The following are some conclusions that have been drawn from the comparative and parametric studies using MADM (i.e., FOBT, SOBT^{1,2}) for the different cases.

- (i) Results from SOBT^{1,2} are closer to the results of Salpekar *et al* (1988), Sela *et al* (1989) and Chatterjee (1991) while results of SOBT and FOBT are relatively less close.
- (ii) Compliance and SERR increase as crack length increases for all beam theories.
- (iii) The SERR values from SOBT² theory approach SERR values from FOBT as the crack length increases.
- (iv) As $\frac{L}{h}$ ratio increases, compliance and SERR increase for FOBT & SOBT^{1,2} beam theories.

The governing differential equations, for the SOBT & FOBT, have been derived for unidirectional laminated composites using principle of MADM and variational principles. Griffith's crack growth criterion and compliance approach have been presented to determine the SERR.

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